# Introduction to SOIL MECHANICS







### Béla Bodó & Colin Jones





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#### About the companion website

This book's companion website is at <u>www.wiley.com/go/bodo/soilmechanics</u> and offers invaluable resources for students and lecturers:

- Supplementary problems
- Solutions to supplementary problems



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Béla Bodó and Colin Jones

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### Preface

This book is intended to introduce the subject to students studying for BTEC Higher National Certificate/Diploma in Civil Engineering and Building Studies or for a Degree in Civil Engineering. It should also be practical reference to Architects, Geologists, Structural and Geotechnical Technicians.

The primary aim is to provide a clear understanding of the basic concepts of Soil Mechanics. We endeavoured to avoid the temptation of over-elaboration by providing excessively detailed text, unnecessary at this early stage of technical studies.

The purpose of this publication is threefold:

- 1. To introduce the student to the basics of soil mechanics.
- 2. To facilitate further advanced study.
- 3. To provide reference Information.

In order to satisfy the above requirements, the concepts of the subject are defined concisely, aided by diagrams, charts, graphs, tables and worked examples as necessary.

The text may appear to be excessively analytical at first sight, but all formulas are derived in terms of basic mathematics, except for a few requiring complicated theory, for those interested in working from first principles. They can be applied however, without reference to the derivation. The expressions are numbered and referred to throughout the text.

There are numerous worked examples on each topic as well as supplementary problems. All examples and problems are solved, many of them interrelated so that solutions can be compared and verified by means of several methods. Some soil testing procedures are outlined only, as there are a number of excellent, detailed, specialized books and laboratory manuals available to cover this part of the subject.

There is some emphasis on the units employed and on the difference between mass and weight. This subject is discussed in Appendix A.

Béla Bodó and Colin Jones

### Dedication

"I dedicate this book to my late wife Dorie."

Béla Bodó

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### **List of Symbols**

CBR California bearing ratio

- C<sub>r</sub> Relative compaction
- D<sub>r</sub> Relative Density
- e Voids ratio
- G<sub>S</sub> Specific gravity
- k CBR Load-ring factor
- M Total Mass of sample
- *m* Moisture (water) content
- *m*<sub>O</sub> Optimum moisture content
- *M*<sub>S</sub> Mass of solids
- $M_{\rm W}$  Mass of water
- n Porosity
- P CBR applied force
- Pa Percentage of air voids
- Q CBR Load gauge reading
- S<sub>r</sub> Degree of saturation
- V Total volume of sample
- Va Volume of air
- V<sub>C</sub> Volume of calibrating cylinder
- *V*<sub>S</sub> Volume of solids
- $V_V$  Volume of voids

- $V_{\rm W}$  Volume of water
- W Total weight of sample
- $W_{S}$  Weight of solids
- $W_W$  Weight of water
- $\delta$  CBR Penetration distance (delta)
- $\gamma$  Bulk weight density (Gamma)
- $\gamma$  Submerged weight density
- γ<sub>d</sub> Dry Weight density
- $\gamma_{d}$  Dry Unit weight to be achieved by compaction
- $\gamma_{S}$  Weight density of solids
- $\gamma_{sat}$  Saturated weight density
- $\rho$  Bulk mass density
- $\rho_{\rm d}$  Dry mass density
- $\rho_{sat}$  Saturated mass density
- $\rho'$  Submerged mass density
- $\rho_{\rm S}$  Mass density of solids

- C<sub>d</sub> Correction for dispersing agent
- C<sub>m</sub> Meniscus correction
- D Equivalent particle diameter
- $D_{10}$  Effective size of a particle
- f Specific Volume change
- *H* Height from the top of the bulb to surface
- *h*b Length of bulb
- $H_{R}$  Height of centre of bulb to surface
- LI Liquidity index
- LL Liquid limit
- $M_{\rm D}$  Mass passing the  $n^{\rm th}$  sieve
- $M_{\rm r}$  Mass retained on the  $n^{\rm th}$  sieve
- $m_{\rm T}$  Temperature correction
- *N* Number of blows
- PI Plasticity index
- PL Plastic limit
- $P_{\rm n}$  Percentage of soil passing the  $n^{\rm th}$  sieve
- R Mixing ratio
- R'h Recorded hydrometer reading
- Rh Corrected hydrometer reading

- RI Relative consistence index
- SL Shrinkage limit
- T Temperature
- t Time
- U Uniformity coefficient
- *u* Velocity of sedimentation
- *V*b Volume of hydrometer bulb
- *V*<sub>O</sub> Volume of over-dried specimen
- ≈ Volume at SL
- *x* Magnitude of linear shrinkage or swelling
- Z Saturation limit
- $\eta$  Dynamic viscosity <eta>

- A Cross-sectional area of specimen
- a Cross-sectional area of standpipe
- A<sub>S</sub> Cross-sectional area of solids in specimen
- A<sub>V</sub> Cross-sectional area of voids in specimen
- EPL Equipotential line
- FL Flow Line
- *F*<sub>S</sub> Factor of safety
- GL Ground level
- GWL Groundwater level (Water Table)
- h Head loss
- $H_{\rm T}$  Total head at x
- $H_X$  Head loss to point x
- $h_{X}$  Pressure head at x
- *i* Hydraulic gradient
- *i*av Average hydraulic gradient
- *i*<sub>C</sub> Critical hydraulic gradient
- *i*e Exit gradient
- k Coefficient of permeability
- L Length of flow path
- *N*e Number of squares (head drops)
- *N*f Number of flow channels

- $N_X$  Number of head drops to point x
- P Hydrostatic force
- Q Flowrate
- q Quantity of flow in time (t)
- *R* Radius of influence
- *r* Radius to observation well
- ro Radius of central well
- *S* Seepage force
- $u_X$  Seepage pore pressure at x
- $\Delta h$  Head Loss between equipotential line
- v Discharge velocity
- *v*<sub>S</sub> Seepage velocity

- / Influence factor
- *n* Number of elements on the Newmark chart
- Q Concentrated point load
- q Uniformly distributed load (UDL)
- r Radius
- z Depth
- $\sigma$  Horizontal pressure
- $\sigma_{\rm V}$  Vertical pressure
- $\tau$  Shear stress

- *dh* Total deformation of specimen of thickness *h*
- h<sub>A</sub> Artesian pressure head
- h<sub>C</sub> Capillary head
- *h*<sub>S</sub> Seepage pressure head
- *i*<sub>C</sub> Critical hydraulic gradient
- *m*<sub>E</sub> Equilibrium moisture content
- mo Optimum moisture content
- pF Soil suction index
- PI Plasticity index
- S<sub>r</sub> Degree of saturation
- S<sub>S</sub> Soil suction
- T Surface tension
- *u* Pore pressure
- $u_{\rm CS}$  Pore pressure in the capillary fringe
- $u_{\rm h}$  Static pore pressure at depth h
- u<sub>S</sub> Seepage pore pressure
- *z*<sub>C</sub> Critical depth for piping
- $\Delta u$  Small change in u
- $\Delta \gamma$  Change in unit weight
- $\Delta\sigma$  Small change in  $\sigma$
- $\Delta\sigma'$  Small change in  $\sigma'$

- $\delta$  Deformation of specimen at time t
- $\sigma$  Total pressure
- $\sigma'$  Effective pressure
- $\sigma_A$  Artesian pressure

- A Pore pressure coefficient
- Pore pressure coefficient
- *B* Pore pressure coefficient
- c Cohesion
- *c*<sub>U</sub> Undrained shear strength
- CD Consolidated-drained test
- CU Consolidated-undrained test
- ESP Effective stress path
- NCC Normally consolidated clay
- *n* Proving ring constant
- OCC Over consolidated clay
- p&q Stress path coordinates
- $p_{f} \& q_{f}$  Stress path coordinates at failure
- QU Quick-undrained test
- $r_X$  Force dial reading at x
- TSP Total stress path
- UU Unconsolidated-undrained test
- x Strain gauge reading
- $\Delta u_{d}$  Change in pore pressure due to  $\Delta \sigma_{d}$
- $\Delta u_{\rm C}$  Change in pore pressure due to  $\Delta \sigma_{\rm C}$
- $\Delta \sigma_{\rm C}$  Change in cell pressure

- $\Delta \sigma_{d}$  Change in the deviator stress
- ε Strain at x
- $\varphi$  Angle of friction
- $\sigma_n$  Normal pressure
- $\sigma_X$  Deviator stress at x
- $\sigma_{\rm U}$  Unconfined compression strength
- $\tau$  Shear stress
- *τ*f Shear stress at failure
- $\tau_{p}$  Shear stress on a plain
- $\tau_{\rm m}$  Maximum shear stress

- A<sub>C</sub> Area indicating completed consolidation
- At Area under an isochrone
- *a*<sub>V</sub> Coefficient of compressibility
- $C_{\alpha}$  Coefficient of Secondary settlement () to consolidation
- *C*<sub>C</sub> Compression index
- C<sub>V</sub> Coefficient of consolidation
- $D_X$  Dial reading at stage x
- dH<sub>i</sub> Initial settlement
- *E* Modulus of elasticity
- e0 Initial voids ratio
- ef Final voids ratio
- es Voids ratio after swelling
- e<sub>x</sub> Voids ratio at stage x
- H Layer thickness
- H<sub>0</sub> Flow path
- $h_X$  Height of specimen at stage x
- *Ip* Influence factor
- k Coefficient of permeability
- $m_V$  Coefficient of volume change
- OCR Overconsolidation ratio

- *q* Bearing pressure
- $T_V$  Time factor
- t Time
- U Average degree of consolidation
- U<sub>Z</sub> Degree of consolidation
- *u* Pore pressure at time t
- *u*<sub>0</sub> Initial pore pressure
- $\Delta H$  Long-term consolidation settlement
- $\Delta \sigma$  Effective consolidating pressure
- $\delta$  Depth factor (Delta)
- $\mu$  Poisson's ratio (My)
- $\sigma_X$  Effective pressure at stage x

c <sub>u</sub>	Unconfined compression strength
сW	Adhesion between soil and wall
е	Eccentricity
Fφ	Factor of safety in terms of friction angle
f <sub>max</sub>	Maximum compressive stress
<i>f</i> min	Minimum compressive stress
F <sub>S</sub>	Factor of safety
Н	Height of wall
H <sub>0</sub>	Height of unsupported clay
Κ	Coefficient of lateral pressure
К0	Coefficient of earth pressure at rest
Ka	Coefficient of active earth pressure
Kf	Coefficient of earth pressure at failure
Кр	Coefficient of passive earth pressure
L	Length of slip surface
<i>M</i> max	Maximum bending moment
<i>M</i> <sub>0</sub>	Overturning moment
M <sub>R</sub>	Resisting moment
Pa	Active force
Pp	Passive force

- *P*<sub>W</sub> Force of water in tension crack
- *R* Force on wedge
- *T* Tension force in tie rod
- *z*<sub>C</sub> Pile penetration
- *z*<sub>0</sub> Depth of tension crack
- $\delta$  Angle of wall friction
- $\varphi'_{m}$  Mobilised friction
- $\mu$  Coefficient of friction
- $\sigma_a$  Active earth pressure
- $\sigma_{\rm C}$  Cell pressure in triaxial test
- $\sigma_{d}$  Deviator stress in triaxial test
- $\sigma_{\rm D}$  Passive earth pressure
- $\sigma_{a}$  Effective active earth pressure
- $\sigma_{p}$  Effective passive earth pressure
- $\sigma'_{p}$  Average pressure
- $\tau_{\rm f}$  Shear stress at failure